

DEPARTMENT OF TRANSPORTATION

DIVISION OF ENGINEERING SERVICES

OFFICE ENGINEER

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*Serious Drought.
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July 25, 2016

11-SD-8, 163-2.4, 3.7
11-416804
Project ID 1113000121
ACHSNHPI-008-1(320)E

Addendum No. 1

Dear Contractor:

This addendum is being issued to the contract for CONSTRUCTION ON STATE HIGHWAY IN SAN DIEGO COUNTY IN SAN DIEGO AT ROUTE 8/163 SEPARATION to revise the project plans, the *Notice to Bidders and Special Provisions*, the *Bid* book and the Federal Minimum Wages with Modification Number 8 dated 07/22/2016.

Submit bids for this work with the understanding and full consideration of this addendum. The revisions declared in this addendum are an essential part of the contract.

Bids for this work will be opened on Wednesday, August 10, 2016.

Project plan sheets 14 and 41 are replaced and attached for substitution for the like-numbered sheets.

In the Special Provisions, Section 37-6, is replaced as attached.

In the Special Provisions, Section 39, is replaced as attached.

In the *Bid* book, in the "Bid Item List," Item 66 is replaced.

To *Bid* book holders:

Inquiries or questions in regard to this addendum must be communicated as a bidder inquiry and must be made as noted in the *Notice to Bidders* section of the *Notice to Bidders and Special Provisions*.

Submit the *Bid* book as described in the *Electronic Bidding Guide* at the Bidders' Exchange website.

http://www.dot.ca.gov/hq/esc/oe/electronic_bidding/electronic_bidding.html

Inform subcontractors and suppliers as necessary.

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This addendum, EBS addendum file, attachments and the modified wage rates are available for the Contractors' download on the Web site:

http://www.dot.ca.gov/hq/esc/oe/project_ads_addenda/11/11-416804

If you are not a *Bid* book holder, but request a book to bid on this project, you must comply with the requirements of this letter before submitting your bid.

Sincerely,



LAURIE BERMAN
District Director

Attachments

Add to section 37:

37-6 HIGH FRICTION SURFACE TREATMENT

37-6.01 GENERAL

37-6.01A Summary

Section 37-6 includes specifications for applying high friction surface treatment (HFST).

Applying HFST consists of spreading resin binder and calcined bauxite aggregate on asphalt concrete surfaces.

HFST may be applied by either hand, mechanical or automated application process. The Department is aware of at least one automated continuous application method and equipment that is allegedly covered by United States Patents 9,109,332 and 9,115,473.

37-6.01B Definitions

Resin binder: a polymeric resin binder used to bond a surface applied aggregate to an asphalt concrete surface.

Prime coat: a polymeric resin that is used to fill cracks and voids in existing surface that is compatible with resin binder.

37-6.01C Submittals

37-6.01C(1) General

Submit the names of your proposed independent laboratories that will perform QC testing.

Submit a certificate of compliance and certified test results for the resin binder and calcined bauxite aggregate. Test results must be from tests performed within 90 days from the date of submittal and must have been performed by an independent laboratory.

Submit proof of recent Dynamic Friction Tester calibration meeting manufacturer's recommendations not to exceed 1 year.

Submit a SDS for the resin binder and its components.

37-6.01C(2) Quality Control Plan

Submit a QC plan that must be project specific and includes:

1. Surface preparation methods for areas where HFST is to be placed
2. Method of protecting areas and exposed facilities not to receive HFST
3. Type of resin binder to be used
4. Resin binder manufacturer's recommended mixing and placement instructions, including mixing ratios and temperatures
5. Resin binder manufacturer's estimated cure times for resin binder to be used
6. Method for safe storage and handling of HFST components
7. Disposal methods for excess HFST and containers for HFST components
8. Contingency plan that describes corrective actions you will take in the event of equipment failure or material issues during HFST placement

Submit QC test results for the quality characteristics within the reporting time allowance, after sampling, shown in the following table:

Quality Control Test Result Reporting

Quality characteristic	Test Method	Maximum reporting time allowance
Los Angeles rattler loss at 100 revolutions	California Test 211	2 business Days
Aggregate moisture content	California Test 226	2 business Days
Aggregate magnesium soundness	ASTM C88	7 Days
Aluminum oxide content	ASTM C25	7 Days
Gradation	California Test 202	1 business Day
Polish stone value	ASTM D3319	10 Days
Aggregate acid insolubility	ASTM D3042	7 Days
Resin binder spread rate	Calculated based on amount of materials used	1 business Day
Viscosity	ASTM D2196	2 business Days
Elongation at break point	ASTM D638	10 business Days
Ultimate tensile strength	ASTM D638	10 business Days
Cure rate	ASTM D1640	1 business Day
Gel time	ASTM C881	1 business Day
Adhesive strength at 24 hours	ASTM C1583	2 business Day
Coefficient of friction before opening to traffic	ASTM E1911	Same day of testing and before opening to traffic
Coefficient of friction 7-15 days after opening to traffic	ASTM E1911	1 business Day after testing

37-6.01D Quality Assurance

37-6.01D(1) General

Not Used

37-6.01D(2) Quality Control

37-6.01D(2)(a) General

Perform QC testing for Trial HFST and production work. QC testing, except coefficient of friction testing (ASTM E1911), must be performed by independent laboratories.

37-6.01D(2)(b) HFST

Perform sampling and testing at the specified frequency and sampling location for the following quality characteristics:

Calcined Bauxite Quality Control Requirements

Quality characteristic	Test method	Minimum sampling and testing frequency	Location of Sampling
Los Angeles rattler loss at 100 revolutions	California Test 211	1st day of production	Point of application or stockpile
Aggregate moisture content	California Test 226	1 per shift	Point of application or stockpile
Aggregate magnesium soundness	ASTM C88	1st day of production	Point of application or stockpile
Aluminum oxide content	ASTM C25	1st day of production	Point of application or stockpile
Gradation	California Test 202	1st day of production	Point of application or stockpile

Resin Binder Quality Control Requirements

Quality characteristic	Test method	Minimum sampling and testing frequency	Location of Sampling
Viscosity	ASTM D2196	1 per Day	Point of application
Elongation at break point	ASTM D638	1 per Day	Point of application
Ultimate tensile strength	ASTM D638	1 per Day	Point of application
Cure rate	ASTM D1640	1 per Day	Point of application
Gel time	ASTM C881	1 per Day	Point of application
Adhesive strength at 24 hours	ASTM C1583	1 per Day	Point of application

HFST Quality Control Requirements

Quality characteristic	Test method	Minimum sampling and testing frequency	Location of Sampling
Resin binder spread rate	Calculated based on amount of materials used	1 per Day	Point of application
Coefficient of friction before opening to traffic	ASTM E1911	Minimum of 1 every 500 ft ^a	Alternate between wheel paths
Coefficient of friction 7-15 days after opening to traffic	ASTM E1911	Minimum of 1 every 500 ft ^a	Alternate between wheel paths

^aFor application lengths <500 ft, test at every 200 ft interval

Before the initial testing of the coefficient of friction of HFST, perform a calibration check of the Dynamic Friction Tester in the presence of the Engineer.

Perform coefficient of friction (ASTM E1911) testing in the presence of the Engineer. Notify the Engineer at least 48 hours before coefficient of friction testing.

37-6.01D(3) Department Acceptance

HFST acceptance is based on:

1. Visual inspection for the following:
 - 1.1. Uniform surface texture
 - 1.2. Raveling, which consists of the separation of the aggregate from the resin binder
 - 1.3. Streaking, which consists of alternating longitudinal bands of resin binder without uniform calcined bauxite aggregate retention, approximately parallel with the lane line
 - 1.4. Flushing, which consists of resin binder without or fully embedded calcined bauxite aggregate

Areas of raveling, streaking and flushing that are greater than 0.25 sq ft shall be considered defective and must be repaired at your own cost. These must be removed and replaced, and must confirm to the maximum lateral dimensions of the defective area.

Raveling and streaking must be repaired by removing and replacing. An additional layer of HFST must be placed over the defective area after removing the HFST in defective areas.

2. For calcined bauxite aggregate, acceptance is based on the Department's sampling and testing for compliance with the requirements shown in the following table:

Calcined Bauxite Aggregate Acceptance Criteria

Quality characteristic	Test method	Requirement
Los Angeles rattler loss at 100 revolutions ^a (max, %)	California Test 211	10
Aggregate moisture content (max, %)	California Test 226	0.2
Sand equivalent (min)	California Test 217	95
Gradation (% passing by weight)		
Sieve size:		
No. 4	California Test 202	100
No. 6		95
No. 16		5

^aUse grading D from Table 1.

3. For resin binder, acceptance is based on the Department's sampling and testing for compliance with the requirements shown in the following table:

Resin Binder Acceptance Criteria

Quality characteristic	Test method	Requirement
Viscosity ^a (centipoises) Use ASTM D2556 Appendix X1.1 for spindle selection	ASTM D2196	1,000 - 3,000
Cure rate (max, hrs) Specimen, 0.2 inch thick	ASTM D1640	3
Gel time ^a (minutes)	ASTM C881	7-30
Elongation at break point ^a (min, %) Type I specimen, Cure the specimen for 7 days at 73°F ± 2°F and test without delay	ASTM D638	30
Ultimate tensile strength ^a (min, psi) Type I specimen, Cure the specimen for 7 days at 73°F ± 2°F and test without delay	ASTM D638	2,650

^aPerform the testing at a temperature of 73 ± 2 °F

4. For HFST, acceptance is based on your QC testing for compliance with the requirements shown in the following table:

HFST Acceptance Criteria

Quality Characteristic	Test Method	Requirement
Coefficient of friction at 60 km/h before opening to traffic	ASTM E1911	0.75 ^a
Coefficient of friction at 60 km/h 7-15 days after opening to traffic	ASTM E1911	0.75 ^a

^a Report coefficient of friction values at 20 km/h, 40 km/h, 80 km/h.

37-6.01D(4) Preconstruction Meeting

Schedule a preconstruction meeting with the engineer at a mutually agreed time and place. Make the arrangements for the meeting facility.

You must be prepared to discuss the following:

1. QC plan
2. Trial HFST requirements
3. Application rates
4. Binder resin mixing methods and equipment
5. HFST application methods and equipment

37-6.01D(5) Trial HFST Application

Do not begin trial HFST until authorized.

Complete a trial of HFST application at an authorized location before starting production work.

Remove pavement markers and delineation within the area to receive HFST, for the lane and length involved, prior to placing the resin binder.

The trial HFST application must:

1. Be at least 12 feet wide and 20 feet long.
2. Be constructed using the same method and equipment as the production work. Construct an additional trial for each method proposed for the production work.
3. Replicate field conditions, including ambient and surface temperatures, anticipated for production work.
4. Demonstrate surface preparation requirements as outlined in the QC plan.
5. Document the area of application, initial quantities of resin binder and aggregate, and unused quantities of resin binder and aggregate after applying the HFST. Calculate and report HFST application rate.
6. Determine the initial set time for the resin binder.
7. Test the coefficient of friction using ASTM E1911 at 20 km/h, 40km/h, 60 km/h and 80 km/h on the HFST. If the coefficient of friction at 60 km/h speed is below 0.75, correct or replace the HFST until the coefficient of friction is greater than or equal to 0.75.

Do not begin HFST production work until authorized after successful completion of the trial HFST.

37-6.02 MATERIALS

37-6.02A General

Not Used

37-6.02B Resin Binder

Resin binder must meet the requirements shown in the following table:

Resin Binder Requirements

Quality characteristic	Test method	Requirement
Viscosity ^a (centipoises) Use ASTM D2556 Appendix X1.1 for spindle selection	ASTM D2196	1,000 - 3,000
Cure rate (max, hrs) Specimen, 0.2 inch thick	ASTM D1640	3
Gel time ^a (minutes)	ASTM C881	7-30
Elongation at break point ^a (min, %) Type I specimen, Cure the specimen for 7 days at 73°F ± 2°F and test without delay	ASTM D638	30
Ultimate tensile strength ^a (min, psi) Type I specimen, Cure the specimen for 7 days at 73°F ± 2°F and test without delay	ASTM D638	2,650
Compressive strength (min, psi at 3 hours)	ASTM C695	1,600
Water absorption (max, %)	ASTM D570	1.0
Durometer hardness (Shore D) Cure the specimen for 7 days at 73°F ± 2°F and test without delay	ASTM D2240	65-75
Flexural yield strength	ASTM D790	2,000
Adhesive strength at 24 hours (min, psi)	ASTM C1583	250 or 100% substrate failure

^aPerform the testing at a temperature of 73 ± 2 °F

37-6.02C Calcined Bauxite Aggregate

Calcined bauxite aggregate must be clean, dry, and free from clay and any other deleterious matter and meet the requirements shown in the following table:

Calcined Bauxite Aggregate Requirements

Quality characteristic	Test method	Requirement
Los Angeles rattler loss at 100 revolutions ^a (max, %)	California Test 211	10
Aggregate moisture content (max, %)	California Test 226	0.2
Sand equivalent (min)	California Test 217	95
Polish stone value (min)	ASTM D3319	38
Aggregate acid insolubility (min, %)	ASTM D3042	90
Aggregate magnesium soundness (max, %)	ASTM C88	30
Aluminum oxide content (min, %)	ASTM C25	87
Gradation (% passing by weight) Sieve size:		
No. 4	California Test 202	100
No. 6		95
No. 16		5

^aUse grading D from Table 1.

37-6.03 CONSTRUCTION

37-6.03A General

Not Used

37-6.03B Surface Preparation

37-6.03B(1) General

Protect utilities, utility covers, drainage structures, curbs and other structures within or adjacent to treatment location from HFST materials using methods outlined in the QC plan.

Surfaces must be clean, dry, and free of any dust, oil, debris, organic matter, or any material that may interfere with the bond between resin binder and existing surfaces.

37-6.03B(2) Pavement Surfaces

For pavement surface preparation perform the following before applying resin binder:

1. Remove pavement markers and delineation to a maximum depth of 0.01 foot from the area receiving HFST
2. Sweep the pavement surface
3. Blow the surface clean with pressurized air
4. Clean cracks greater than 0.25 inch wide with pressurized air and pretreat with mixed resin binder

37-6.03C HFST Application

Apply mixed resin binder under resin binder manufacturer's recommendations.

Do not apply resin binder on wet or damp surfaces. Asphalt concrete pavement surface must be greater than 30 days old before applying HFST.

Do not apply HFST when the ambient temperature is below 50 degrees F for epoxy type resin binders and 45 degrees F for other resin binders. Do not apply HFST when the ambient temperature is above 100 degrees F.

Spread resin binder at a minimum rate of 0.32 gal/sq yd to one lane width at a time. Narrower application widths are allowed as determined by the engineer.

Do not allow the mixed resin binder to do any of the following that may impair retention and bonding of aggregate:

1. Separate
2. Cure
3. Dry
4. Be exposed
5. Harden

Do not contaminate the exposed uncured mixed resin binder.

Replace contaminated areas of resin binder.

Spread aggregate until refusal within 5 minutes of resin binder application.

Cure HFST for a minimum period recommended by the resin binder supplier. During curing period do not allow vehicles, construction equipment, or foot traffic on the HFST.

HFST that has not completely cured is considered non performing and must be removed and replaced before opening to traffic.

37-6.03D Excess Aggregate Removal and Reuse

Excess calcined bauxite aggregate must be recovered by a mechanical sweeper and may be reused for HFST. Before reuse of recovered calcined bauxite aggregate, meet the requirements under 37-6.01D(2) and blend with new calcined bauxite aggregate at a rate of 2 to 1 by volume. Provide a record of all recovered calcined bauxite aggregate used and test results. Super sacks or stockpile containing the blended recovered calcined bauxite aggregate must be clearly marked "Recovered Calcined Bauxite Aggregate" and the contract number.

Before opening to public traffic, remove excess and loose aggregate from the traveled way and shoulders by sweeping. HFST must be completely cured before sweeping and there must be no damage or dislodging of aggregate from HFST surface. Perform additional sweeping before placement of pavement markers and delineation.

37-6.04 PAYMENT

Not Used

39 HOT MIX ASPHALT

Add between "single" and "test" in the 8th paragraph of section 39-1.01D(9)(a) of the RSS for section 39:
aggregate and HMA mixture

Replace section 39-1.03K of the RSS for section 39 with:

39-1.03K Rumble Strips

Construct rumble strips in the top layer of HMA surfacing by ground-in methods.

Select the method and equipment for constructing ground-in indentations.

Do not construct rumble strips on structures or approach slabs.

Construct rumble strips within 2 inches of the specified alignment. The grinding equipment must be equipped with a sighting device enabling the operator to maintain the rumble strip alignment.

Indentations must comply with the specified dimensions within 1/16 inch in depth and 10 percent in length and width.

The Engineer orders grinding or removal and replacement of noncompliant rumble strips to bring them within specified tolerances. Ground surface areas must be neat and uniform in appearance.

The grinding equipment must be equipped with a vacuum attachment to remove residue from the roadbed.

Dispose of removed material.

On ground areas, apply fog seal coat under section 37-2.

Add to section 39-1.04 of the RSS for section 39:

Rumble strips are measured by the station along the length of the rumble strips without deductions for gaps between indentations.

Replace the paragraphs in section 39-2.01C(2) of the RSS for section 39 with:

The JMF must be based on the Superpave HMA mix design system as described in the MS-2 Asphalt Mix Design Methods by the Asphalt Institute.

For a Type A HMA mixture using RAP substitution greater than 15 percent of the aggregate blend, the asphalt binder grade from the HMA mixture must comply with the binder grade specified in section 39-2.02C. The HMA mixture binder grade must not be stiffer than the PG binder grade specified and must be determined by blending charts for high, intermediate, and low critical temperatures. Original binder requirements, ductility requirements, and footnote d in the table in the 1st paragraph in section 92-1.02B do not apply in the determination of the HMA mixture binder grade using blending charts.

Add to section 39-2.01C(3) of the RSS for section 39:

For RAP substitution greater than 15 percent of the aggregate blend, submit blending calculation sheets and blending charts for high, intermediate, and low critical temperatures. The blending calculation sheets and blending charts must be based on the MS-2 Asphalt Mix Design Methods by the Asphalt Institute. You may use critical temperatures of virgin binder or the maximum theoretical critical temperature of the PG grade of the virgin binder. Critical temperatures must be in whole degree. The calculation sheets must be sealed and signed by an engineer who is registered as a civil engineer in the State or by the AMRL-AASHTO-accredited laboratory manager responsible for the calculations and blending charts.

Add between the heading and the 1st paragraph of section 39-2.01D(2)(c) of the RSS for section 39:

39-2.01D(2)(c)(i) General

Section 39-2.01D(2)(c) applies to Type A HMA mixtures using RAP substitution greater than 15 percent of the aggregate blend.

39-2.01D(2)(c)(ii) Reclaimed Asphalt Pavement Stockpiles

Add to section 39-2.01D(2)(c) of the RSS for section 39:

39-2.01D(2)(c)(iii) Virgin and Recovered Reclaimed Asphalt Pavement Binder

Perform solvent extraction of RAP binder under AASHTO T 164, Method A, and recovery under AASHTO R 59 or ASTM D1856. Test the quality characteristics of the recovered RAP binder under the test methods and frequencies shown in the following table:

Quality characteristic	Test method	Minimum testing frequency
Critical temperatures of RAP binder	AASHTO T 315 and AASHTO T 313	1 per project if RAP is not augmented or 1 per 500 tons of augmented RAP

If you use critical temperature of virgin binder in blending charts, test the quality characteristics of the virgin binder under the test methods and frequencies shown in the following table:

Quality characteristic	Test method	Minimum testing frequency
Critical temperatures of virgin binder	AASHTO T 315 and AASHTO T 313	1 per 5 paving days or 1 per project, whichever is greater

Determine the blended binder grade using blending charts under the MS-2 Asphalt Mix Design Methods by the Asphalt Institute each time the critical temperatures are determined.

Replace "If RAP is used" in item 2 in the list of the paragraph of section 39-2.01D(5) of the RSS for section 39 with:

For RAP substitution greater than 15 percent of the aggregate blend

Replace the row for moisture susceptibility, dry strength, in the table in item 3 in the list of the paragraph of section 39-2.01D(5) of the RSS for section 39 with:

Moisture susceptibility (psi, dry strength)	AASHTO T 283	100–300
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Add to the list of the paragraph of section 39-2.01D(5) of the RSS for section 39 with:

4. For RAP substitution greater than 15 percent of the aggregate blend, the asphalt binder grade must comply with the specified binder grade. A tolerance of +2 degrees C may be applied to the critical high and low temperatures of the blended binder. Original binder requirements, ductility requirements, and footnote d in the table in the 1st paragraph in section 92-1.02B do not apply in the determination of the PG binder grade using blending charts.

Replace the row for moisture susceptibility, dry strength, in the 1st paragraph of section 39-2.02B of the RSS for section 39 with:

Moisture susceptibility, dry strength (psi)	AASHTO T 283	100–300
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Replace the 3rd and 4th paragraphs in section 39-2.02B of the RSS for section 39 with:

For a Type A HMA mixture using RAP substitution greater than 15 percent of the aggregate blend, the mix design blended binder grade must comply with the specified binder grade. The mix design blended binder grade must be determined using blending charts as described in the MS-2 Asphalt Mix Design Methods by the Asphalt Institute. Original binder requirements, ductility requirements, and footnote d in the table in the 1st paragraph in section 92-1.02B do not apply in the determination of the HMA mixture binder grade using blending charts.

Replace "Reserved" in section 39-2.02C of the RSS for section 39 with:

The grade of asphalt binder for Type A HMA must be PG 64-10.

For Type A HMA using RAP substitution of greater than 15 percent of the aggregate blend, the HMA mixture binder grade must comply with the PG binder grade specified above.

For Type A HMA using RAP substitution of 15 percent or less of the aggregate blend, the grade of the virgin binder must comply with the PG binder grade specified above.

Replace the 2nd sentence of 2nd paragraph in section 39-2.02F of the RSS for section 39 with:

For RAP substitution of 15 percent or less, RAP must be within ± 3 of RAP percentage shown in your Contractor Job Mix Formula Proposal form without exceeding 15 percent. For RAP substitution of greater than 15 percent, RAP must be within ± 3 of RAP percentage shown in your Contractor Job Mix Formula Proposal form without exceeding 25 percent.

**BID ITEM LIST
11-416804**

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity	Unit Price	Item Total
61	031356	FURNISH SINGLE SHEET ALUMINUM SIGN (0.080"-UNFRAMED) FOR RETROREFLECTIVE SHEETING (TYPE XI)	SQFT	98		
62	031357	RETROREFLECTIVE SHEETING (TYPE XI)	SQFT	370		
63	620140	24" ALTERNATIVE PIPE CULVERT	LF	39		
64	703233	GRATED LINE DRAIN	LF	30		
65	721810	SLOPE PAVING (CONCRETE)	CY	10		
66	731508	MINOR CONCRETE (EXPOSED AGGREGATE CONCRETE)	SQFT	840		
67 (F)	750001	MISCELLANEOUS IRON AND STEEL	LB	622		
68	820118	GUARD RAILING DELINEATOR	EA	32		
69	832005	MIDWEST GUARDRAIL SYSTEM	LF	790		
70	839581	END ANCHOR ASSEMBLY (TYPE SFT)	EA	2		
71	839585	ALTERNATIVE FLARED TERMINAL SYSTEM	EA	1		
72	031358	FLARED TERMINAL SYSTEM (TYPE X-TENSION)	EA	1		
73	031359	ALTERNATIVE CRASH CUSHION	EA	1		
74	031360	CONCRETE BARRIER (TYPE 60 MOD)	LF	300		
75	840516	THERMOPLASTIC PAVEMENT MARKING (ENHANCED WET NIGHT VISIBILITY)	SQFT	130		
76	846007	6" THERMOPLASTIC TRAFFIC STRIPE (ENHANCED WET NIGHT VISIBILITY)	LF	1,960		
77	846009	8" THERMOPLASTIC TRAFFIC STRIPE (ENHANCED WET NIGHT VISIBILITY)	LF	990		
78	846010	8" THERMOPLASTIC TRAFFIC STRIPE (ENHANCED WET NIGHT VISIBILITY) (BROKEN 12-3)	LF	140		
79	031361	6" THERMOPLASTIC TRAFFIC STRIPE (ENHANCED WET NIGHT VISIBILITY) (BROKEN 36-12)	LF	890		
80	850101	PAVEMENT MARKER (NON-REFLECTIVE)	EA	75		